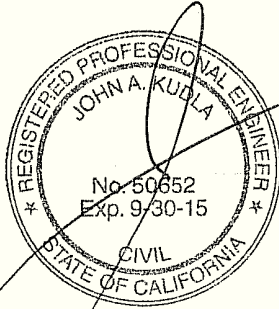


JK ENGINEERING

CIVIL ENGINEERING AND STRUCTURAL DESIGN

610 10TH ST. SUITE A, PASO ROBLES, CA. 93446

Ph. (805) 239-4151 Fax (805) 239-5853



November 16, 2013

Page 1 of 22

Job #: 1113-01-16

CLIENT: Melaine Tucker

PROJECT: Drainage Report

LOCATION: 6805 Rocky Canyon Road, Atascadero CA 93422

The purpose of this flood study is to determine the effects of a metal building used as a dog kennel placed in the 100-yr floodplain. The Department of Public Works for the County of San Luis Obispo requested a determination be made on the possible changes in the floodplain elevation as a result of the presence of the metal structure. The analysis and conclusions within this report are based on a floodway study produced by Federal Emergency Management Agency (FEMA) and a base elevation survey produced by Twin Cities Surveys located in Templeton, California.

Background:

This office produced a structural analysis of the existing metal structure on April 17, 2012. The structure is 16 feet wide, 212 feet long, and average building height of 8 feet. The structure is located at 6805 Rocky Canyon Road in San Luis Obispo County. It is proposed the building be used as a dog kennel. The structural analysis found the building to be adequate to withstand the current California Building Code (CBC) loads for the proposed use.

Twin City Surveys produced an elevation survey of the project area which extends between Rocky Canyon Road and the Salinas River. The survey included the existing building finish floor elevations as well as the areas used for the dog kennel runs adjacent to the structure. The concrete slab finished floor elevation was found to vary from 839.02 to 839.96 feet above sea level. Twin Cities Surveys determined the 100-yr flood elevation to be 843 feet based on the 1929 vertical datum and FEMA's Flood Insurance Rate Map (FIRM). They reported the lowest finish floor elevation is 3.98 feet below the 843 flood elevation. The nearest structure to the kennel is a residence located upstream (southerly) approximately 250 feet and the finish floor elevation is 855.9 feet or approximately 859 on the 1988 vertical datum.

The FEMA 100-yr floodplain elevation used in this report is 846.1 feet and is based on the 1988 vertical datum. Switching to the 1988 vertical datum is common and should not cause confusion. The FEMA flood study was conducted on November 16, 2012 within the Salinas River in this location. The study includes the entire Salinas River. The report number is 06079c0932G and the applicable sections of the report are included in this report. The FEMA report produced a river cross section, flow data, and a river profile near this project location, see cross section AV and profile 64P, attached.

The floodway data shows the width of the floodway is 505 feet, the cross sectional area is 4940 square feet, the mean velocity is 5.4 feet per second, and the flood water-surface elevation based on the 1988 NVAD is 846.1 feet above sea level. This data is used to determine the effects of the 100-yr flow on the structure.

Analysis:

The Department of Public Works questioned the effects on the 100-yr flood surface elevation after the metal structure is placed in the floodplain. One concern is if a hydraulic jump would occur at the face of the structure. A hydraulic jump can raise the water surface elevation as the stream flow passes over an object in the stream flow. This occurs when there is an abrupt change in the stream flow velocity.

In this case a minor hydraulic jump will occur as the stream current hits the structure columns. The height of the jump will be 0.45 ft. This height is less than 1' which is the standard free board height used to account for minor hydraulic jumps and local turbulences caused by debris in the stream flow. See attached calculations for the hydraulic jump determination.

Also, the increased surface elevation should not affect adjacent property owners. The nearest structure is a residence owned by the property owner. The finish floor elevation of the residence is 859 feet and located up stream approximately 250 feet. The maximum elevation of the water surface at flood stage with the hydraulic jump would be 846.55 feet ($846.1 + 0.45$). This is 12.45 feet below the finish floor of structure.

The next structure is located approximately 1000 feet upstream of the dog kennel. It is approximately the same elevation of the owner's residence. From the FEMA report the approximate slope of the stream bed can be determined. At cross section AV the flood elevation is 846.1 feet. At the next upstream cross section AW the elevation is 850 feet. Both cross sections are 2482 feet apart. With this information the average slope of the stream bed is determined to be 0.16% ($(850 - 846.1) / 2482$). Assuming the hydraulic jump caused by an 8" column located in a stream flow that is 500 feet wide is capable of producing a 0.45 foot water surface rise uniformly across the entire stream flow, the slope in the channel prevents the hydraulic jump from reaching the adjacent property owners residence. The channel is sloped at 0.16% and the hydraulic jump is 0.45 high. The horizontal length of the hydraulic jump would dissipate at about 281 feet ($0.45 / 0.0016$) upstream.

The jump is minor because the building wall panels will be removed before the 100-yr storm event. The Project Safety Plan requires all the exterior wall panels shall be removed 8 hours prior to the storm event. Removing the panels can be accomplished easily since the wall panel which includes the metal sheathing and wall girts is one complete assembly attached the building columns. The panels are attached to the columns with 4 bolts. The structure frames and tie rods are the only structural components required to support the roof structure. Essentially, after the wall panels are removed the building will be a open structure. The chain link fencing used around the dog runs is not attached to the structure which will not hinder the wall panel removal.

JOHN A. KUDLA , R.C.E. # 50652
PRINCIPAL ENGINEER/ PRESIDENT

Additionally, The Department of Public Works was concerned about the effects of the stream flow forces on the structure. With the wall panels removed the only structural components within the floodplain are the frame columns. According to the structural plans the columns are 8"x3.5" C. From the FEMA report the average stream velocity is 5.4 feet per second. From the Caltrans "Bridge Design Specification", 2004, section 3.18, the design pressure on an object placed in a stream flow is determined by the equation, $P = KV^2$; where P is in pounds per square feet, K is a constant, and V is the average stream velocity in feet per second. The maximum value for P shall not be more than twice the average calculated pressure (P-avg).

In this case, the stream flow force on the structure is determined to be 40.8 pounds per square foot ($1.4 * 5.4^2$). Assuming the stream flow hits the entire column height of 8 feet, the lateral force on the column is 218.7 pounds. See attached calculations within this report. From the structural analysis performed by this office, the seismic and wind forces placed on the building on structural building lines 1 and line 2, are 368 and 984 pounds, respectfully. See sheet 26 of the original structural calculation which is included in this report. The design loads used in the structural analysis are greater than the stream flow forces.

Conclusion and Recommendations:

As shown in the analysis above, the dog kennel structure should have minimal effect on the 100-yr floodplain in this location. The structure columns will produce a minor hydraulic jump but the minor jump is less than the standard free board height of 1 foot. The adjacent property owner located more than 1000 feet upstream from the dog kennel will not be affected by the 281 foot long hydraulic jump. The seismic and wind forces used in the structural analysis are larger than the stream flow forces. It is our opinion, the existing structure will perform adequately for the proposed use.

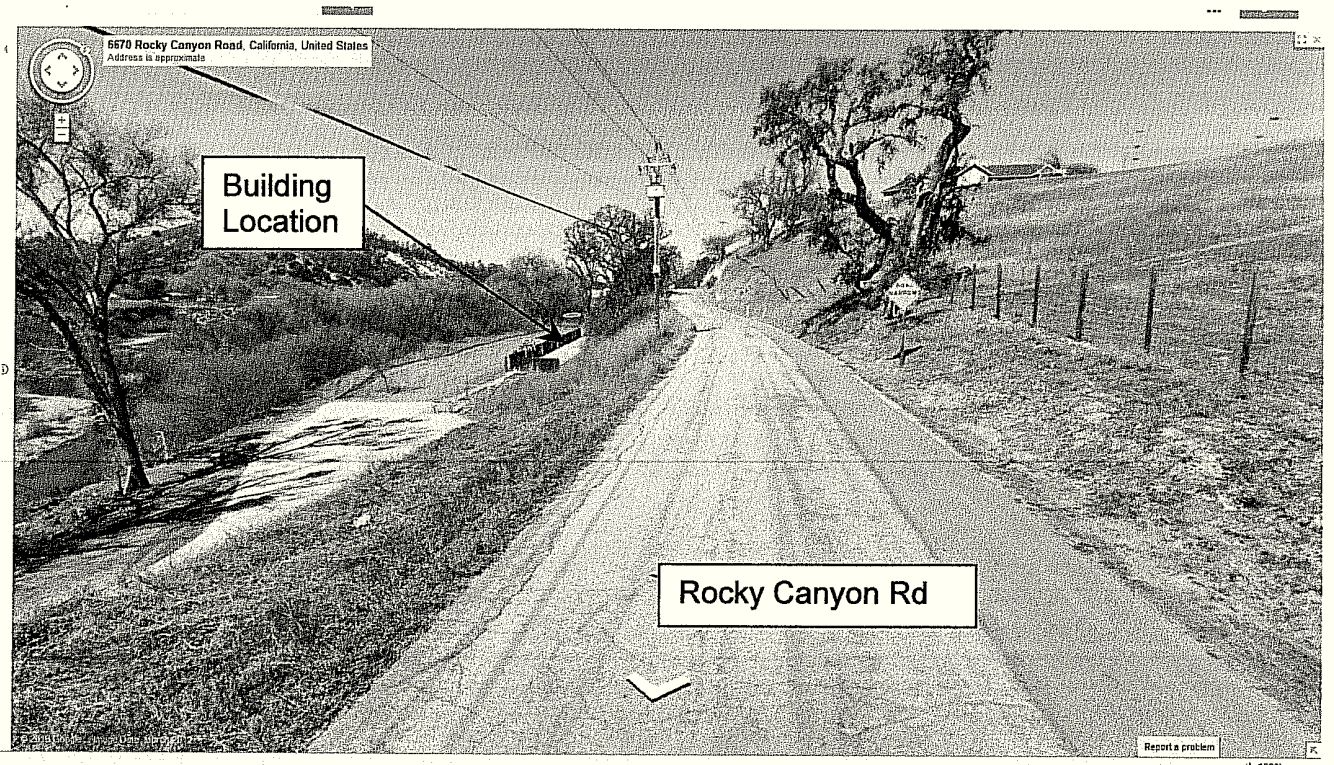
It is recommended the Safety Plan which is attached to this report be on followed closely. The Safety Plan provides evacuation procedures during a major storm event. It is the responsibility of the current owner as well as future owners to adhere to the Safety Plan and the recommendation stated in this report.

Respectfully,

John A. Kudla, RCE 50652
Owner and Principal Engineer.

Attachments: Project Safety Plan

JOHN A. KUDLA , R.C.E. # 50652
PRINCIPAL ENGINEER/ PRESIDENT



Project Location

JOHN A. KUDLA , R.C.E. # 50652
PRINCIPAL ENGINEER/ PRESIDENT

Project: TUCKER KENNEL
FLOOD STUDY Date 11/9/13

CHECK HYDRAULIC JUMP

HEIGHT OF JUMP $h = V^2/2g$, $V = 5.4 \text{ FT/SEC}$ FROM FEMA FLOOD STUDY
 $g = 32.2 \text{ FT/SEC}^2$

$$h = 5.4^2 / 2(32.2) = 0.45' < 1 \text{ FT. OKAY}$$

THE HEIGHT IS LESS THE 1'-FREE BOARD USED TO
ACCOUNT FOR LOCAL TURBULANCE & MINOR HYD. JUMPS

CHECK THE FORCE ON THE COLUMNS

PER "CAUTIONS" BRIDGE DESIGN SPEC., 2004

$$P_{\text{WIND}} = K V_{\text{AVG}}^2, \quad K = 1/4 \text{ FOR SQUARE COLUMNS IN}$$

THE STREAM FLOW, $V^2 = 5.4^2 = 29.7$,

$$P_{\text{WIND}} = 29.7(1/4) = 7.425 \text{ PSF}$$

FROM STR. PLAN SHET S-3 THE BLD'G COLS ARE 8" x 3 1/2" C

ASSUME P_{WIND} ALONG THE ENTIRE COLUMN HGT (8')

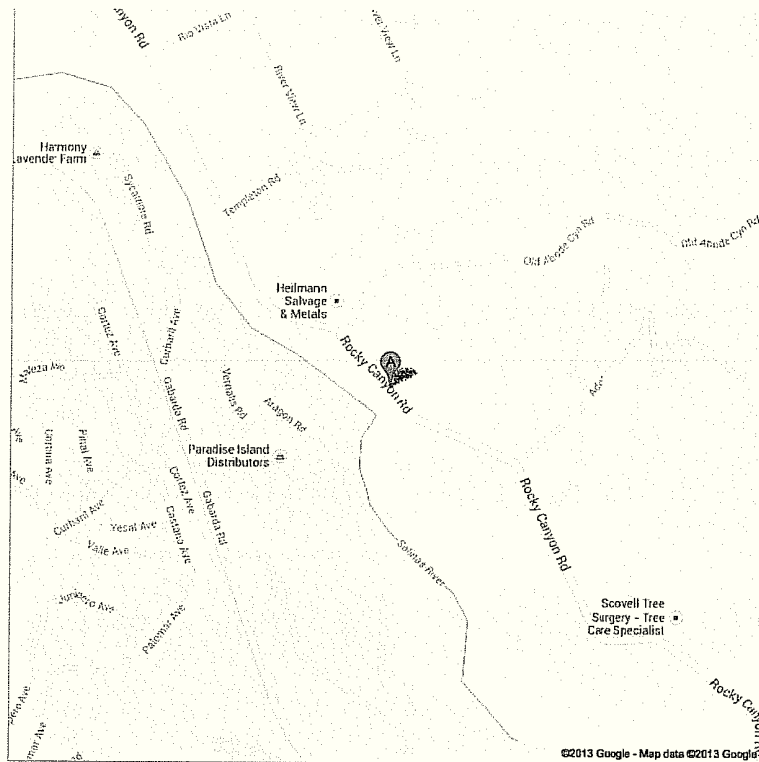
$$A = 8'(0.67') = 5.36 \text{ FT}^2, \quad F = 7.425(5.36) = 39.8 \text{ LBS}$$

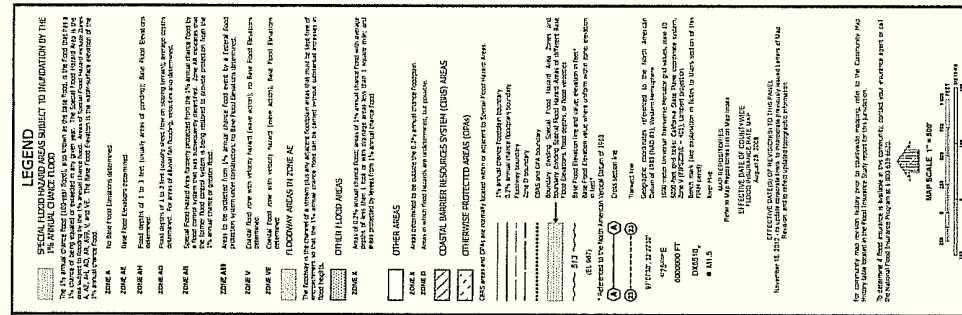
$$\text{FROM STRUCT. CALLS } \left. \begin{array}{l} F_{\text{WIND}} = 368 \text{ LBS} \\ F_{\text{SEIS}} = 984 \text{ LBS} \end{array} \right\} \text{ SEE PG 26/34 LINE 1 \& 2 OF CALLS}$$

$F < F_{\text{WIND}} \& F = F_{\text{SEIS}} \therefore$ FRAME OKAY FOR FORCE
OF STREAM CURRENT.

Digitized by Google

Address **6805 Rocky Canyon Rd**
Atascadero, CA 93422

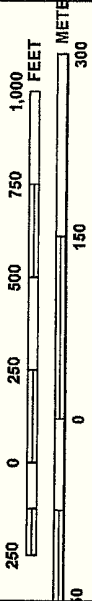


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NFIP NATIONAL FLOOD INSURANCE PROGRAM	PANEL 8333G	FLOOD INSURANCE RATE MAP SAN LUIS OBISPO COUNTY, CALIFORNIA AND INCORPORATED AREAS	PANEL 832 OF 2050 (SEE MAP INDEX FOR FIRM PANEL LOCATION)	CITY/STATE SAN LUIS OBISPO, CALIF. MANUFACTURED BY FEDERAL BUREAU OF SURVEYING	DATE NOVEMBER 1972	SCALE 1" = 1 MILE	MAP NUMBER 060750B33G	DATE REVISSED NOVEMBER 16, 2012		Federal Emergency Management Agency
	<small> Map is made by the Insurance Policy Plan Board, Inc. and is not to be used for any purpose not intended by the Insurance Policy Plan Board, Inc. </small>									



MAP SCALE 1" = 500'



NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0832G

FIRM

FLOOD INSURANCE RATE MAP
SAN LUIS OBISPO
COUNTY,
CALIFORNIA
AND INCORPORATED AREAS

PANEL 832 OF 2050

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
ATASCADERO, CITY OF	060700	0832	G
SAN LUIS OBISPO COUNTY	060304	0832	G

Notice to User: The Map Number shown below should be used when placing map orders, the Community Number shown above should be used on insurance applications for the subject community.



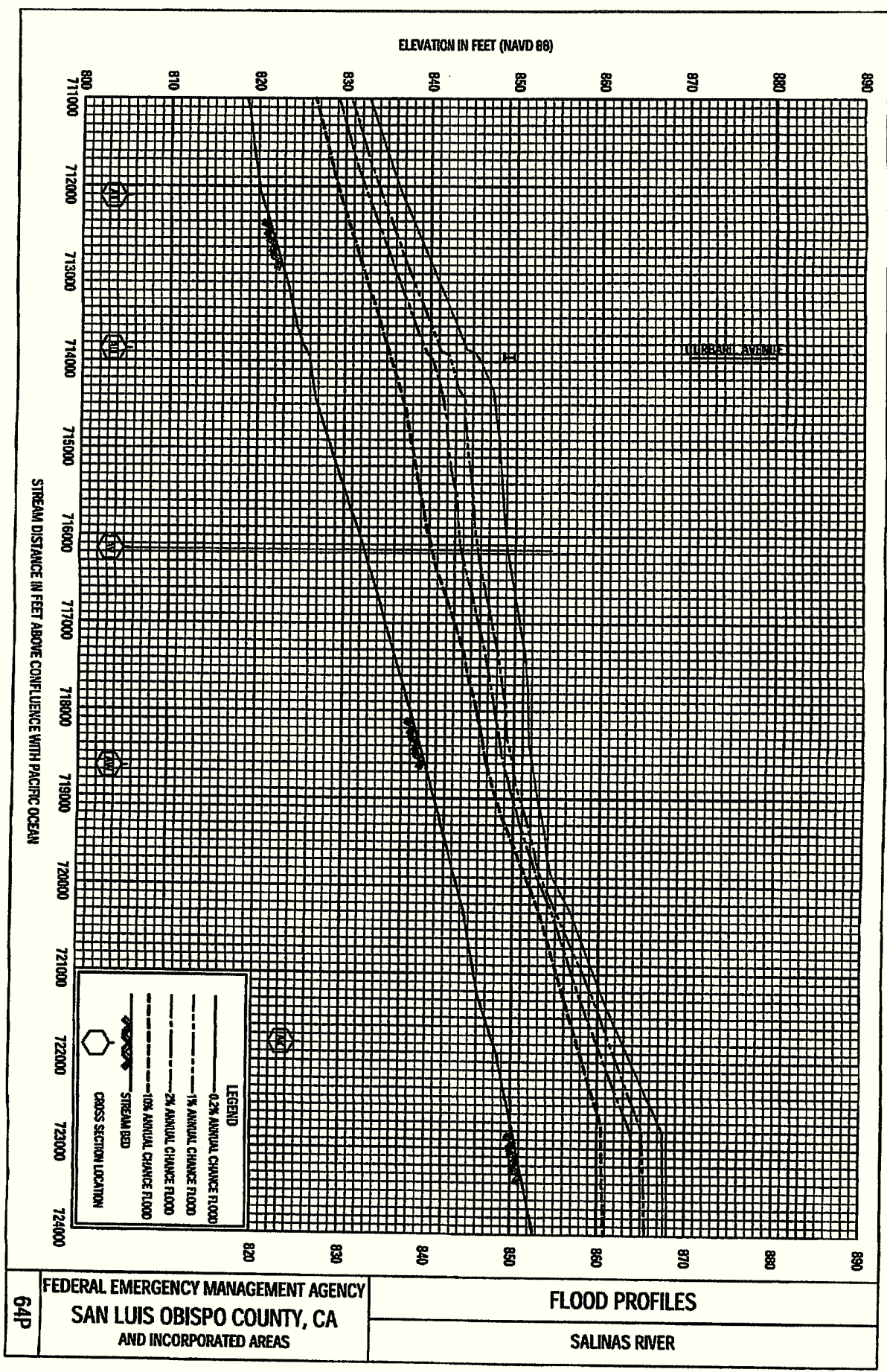
MAP NUMBER
06079C0832G

MAP REVISED
NOVEMBER 16, 2012

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov

9/



FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD 88)			
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Salinas River (continued)								
AA	675,206	1,044	10,595	4.0	746.8	746.8	747.2	0.4
AB	677,477	956	8,871	4.7	750.7	750.7	750.9	0.2
AC	679,536	677	6,314	6.6	755.8	755.8	756.1	0.3
AD	681,437	565	7,228	5.8	759.6	759.6	760.3	0.7
AE	682,334	499	5,627	7.5	761.3	761.3	761.9	0.6
AF	684,288	588	7,265	5.8	766.8	766.8	767.3	0.5
AG	686,347	760	8,918	4.7	770.1	770.1	771.1	1.0
AH	688,090	779	8,499	4.9	773.1	773.1	773.7	0.6
AI	689,832	685	5,727	5.9	776.0	776.0	776.4	0.4
AJ	691,944	923	6,900	4.9	780.9	780.9	781.2	0.3
AK	694,056	612	5,182	6.4	787.7	787.7	787.9	0.2
AL	696,432	1,325	9,488	3.4	792.9	792.9	793.0	0.1
AM	698,597	1,408	8,305	3.7	797.1	797.1	797.2	0.1
AN	700,498	1,107	6,762	4.5	801.1	801.1	802.0	0.9
AO	702,451	946	6,472	4.7	808.0	808.0	808.9	0.9
AP	704,299	852	6,310	4.7	814.1	814.1	814.4	0.3
AQ	705,989	664	5,661	5.2	817.7	817.7	818.2	0.5
AR	708,048	932	5,965	4.9	821.0	821.0	821.4	0.4
AS	709,685	590	4,521	6.2	826.4	826.4	826.4	0.0
AT	712,114	358	4,118	6.8	834.2	834.2	834.5	0.3
AU	713,856	224	2,910	9.6	841.3	841.3	841.6	0.3
AV	716,179	505	4,940	5.4	845.7	845.7	846.1	0.4
AW	718,661	1,810	8,443	3.1	849.9	849.9	850.0	0.1
AX	721,776	390	3,932	5.9	861.7	861.7	861.7	0.0
AY	724,099	1,267	7,793	3.0	865.6	865.6	865.6	0.0
AZ	726,211	1,356	7,241	3.1	869.0	869.0	869.0	0.0
¹ Feet above confluence with Pacific Ocean								
FEDERAL EMERGENCY MANAGEMENT AGENCY					FLOODWAY DATA			
SAN LUIS OBISPO COUNTY, CA AND INCORPORATED AREAS					SALINAS RIVER			
TABLE 9								



3.15.3 Overturning Forces

The effect of forces tending to overturn structures shall be calculated under Groups II, III, V, and VI of Article 3.22 assuming that the wind direction is at right angles to the longitudinal axis of the structure. In addition, an upward force shall be applied at the windward quarter point of the transverse superstructure width. This force shall be 20 pounds per square foot of deck and sidewalk plan area for Group II and group V combinations and 6 pounds per square foot for Group III and Group VI combinations.

3.16 THERMAL FORCES

Provisions shall be made for stresses or movements resulting from variations in temperature. The rise and fall in temperature shall be fixed for the locality in which the structure is to be constructed and shall be computed from an assumed temperature at the time of erection. Due consideration shall be given to the lag between air temperature and the interior temperature of massive concrete members or structures.

The range of temperature shall generally be as follows:

Air Temperature Range	Design Range	
	Steel	Concrete
Extreme: 120° F Certain mountain and desert locations	Rise or Fall 60° F Movement/Unit Length .00039	Rise or Fall 40° F Movement/Unit Length .00024
Moderate: 100° F Interior Valleys and most mountain locations	Rise or Fall 50° F Movement/Unit Length .00033	Rise or Fall 35° F Movement/Unit Length .00021
Mild: 80° F Coastal Areas, Los Angeles, and San Francisco Bay Area	Rise or Fall 40° F Movement/Unit Length .00026	Rise or Fall 30° F Movement/Unit Length .00018

Provisions shall be made in concrete structures for stresses and movements resulting from shrinkage, as follows:

Arches - Shrinkage Coefficient 0.0002

Prestressed Structures - See Division 1, Section 9 of this volume.

Other Structures - The temperature fall from a normal temperature as given above provides adequately for stress and movement caused by shrinkage.

3.17 UPLIFT

Provision shall be made for adequate attachment of the superstructure to the substructure by ensuring that the calculated uplift at any support is resisted by tension members engaging a mass of masonry equal to the largest force obtained under one of the following conditions:

- 100 percent of the calculated uplift caused by any loading or combination of loadings in which the live plus impact loading is increased by 100 percent.
- 150 percent of the calculated uplift at working load level.
- 100 percent of the calculated uplift for load factor design.

Anchor bolts subject to tension or other elements of the structures stressed under the above conditions shall be designed at 150 percent of the allowable basic stress.

3.18 FORCE FROM STREAM CURRENT, FLOATING ICE AND DRIFT

All piers and other portion so structures that are subject to the force of flowing water, floating ice, or drift shall be designed to resist the maximum stresses induced thereby.

3.18.1 Force of Stream Current on Piers

3.18.1.1 Stream Pressure

The effect of flowing water on piers and drift build-up, assuming a second degree parabolic velocity distribution and thus a triangular pressure distribution, shall be calculated by the formula:

$$P_{avg} = K(V_{avg})^2 \quad (3-4)$$



where:

- P_{avg} = average stream pressure, in pounds per square foot;
 V_{avg} = average velocity of water in feet per second; computed by dividing the flow rate by the flow area,
 K = a constant, being 1.4 for all piers subjected to drift build-up and square-ended piers, 0.7 for circular piers, and 0.5 for angle-ended piers where the angle is 30 degrees or less.

The maximum stream flow pressure, P_{max} , shall be equal to twice the average stream flow pressure, P_{avg} , computed by Equation 3-4. Stream flow pressure shall be a triangular distribution with P_{max} located at the top of water elevation and a zero pressure located at the flow line.

3.18.1.1.2 The stream flow forces shall be computed by the product of the stream flow pressure, taking into account the pressure distribution, and the exposed pier area. In cases where the corresponding top of water elevation is above the low beam elevation, stream flow loading on the superstructure shall be investigated. The stream flow pressure acting on the superstructure may be taken as P_{max} with a uniform distribution.

3.18.1.2 Pressure Components

When the direction of stream flow is other than normal to the exposed surface area, or when bank migration or a change of stream bed meander is anticipated, the effects of the directional components of stream flow pressure shall be investigated.

3.18.1.3 Drift Lodged Against Pier

Where a significant amount of drift lodged against a pier is anticipated, the effects of this drift buildup shall be considered in the design of the bridge opening and the bridge components. The overall dimensions of the drift buildup shall reflect the selected pier locations, site conditions, and known drift supply upstream. When it is anticipated that the flow area will be significantly blocked by drift buildup, increases in high water elevations, stream velocities, stream flow pressures, and the potential increases in scour depths shall be investigated.

3.18.2 Force of Ice on Piers

Pressure of ice on piers shall be calculated at 400 pounds per square inch. The thickness of ice and height at which it applies shall be determined by investigation at the site of the structure.

3.19 BUOYANCY

Buoyancy shall be considered where it affects the design of either substructure, including piling, or the superstructure.

3.20 EARTH PRESSURE

3.20.1 Structures which retain level fills shall be proportioned to withstand pressure as given by Rankine's formula, or by other expressions given in Section 5, "Retaining Walls"; provided, however, that no vertical wall structure shall be designed for less than an equivalent fluid unit weight of 36 pounds per cubic foot, except that the maximum load on the heels of wall footings shall be determined by using an equivalent fluid unit weight of 27 pounds per cubic foot.

3.20.2 For rigid frames a maximum of one-half of the moment caused by earth pressure (lateral) may be used to reduce the positive moment in the beams, in the top slab, or in the top and bottom slab, as the case may be.

3.20.3 When highway traffic can come within a horizontal distance from the top of the structure equal to one-half its height, the pressure shall have added to it a live load surcharge pressure equal to not less than 2 feet of the earth.

3.20.4 Where an adequately designed reinforced concrete approach slab supported at one end by the bridge is provided, no live load surcharge need be considered.

3.20.5 All designs shall provide for the thorough drainage of the back-filling material by means of weep holes and crushed rock, pipe drains or gravel drains, or by perforated drains.



ROBERT & MELANIE TUCKER
3155 CRESTON ROAD
PASO ROBLES, CA 93446

[illegible]

These drawings are for inclusion in the files of J.C. Engineering and which he could submit for the purpose of this project on his own. Any one other than the project team which it is intended for without the written consent of J.C. Engineering

SCALE: 1" = 10'-0"



ROOF FRAMING PLAN CALLOUTS

1. 26 GAUGE MULTI-RIB METAL ROOFING W/ #8 SCREWS AT 6" O/C AT EDGES AND 12" O/C IN FIELD.
2. 8"x2.5" Z 16 GAUGE PURLIN (TYP).
3. 4"x2.5" Z 16 GAUGE GIRT (TYP).
4. 8"x3.5" C 16 GAUGE EAVE STRUT (TYP).
5. 8"x3.5" C 16 GAUGE GIRDER (TYP).
6. 8"x3.5" C 16 GAUGE COLUMN (TYP).
7. L 6x6x3/16" A36 BRACKET AT ALL PURLIN TO GIRDER CONNECTIONS.
BRACKETS SHALL BE ANCHORED TO BOTH PURLIN AND GIRDER W/ (2) 1/2" Ø A307 BOLTS WITH EDGE DISTANCES TO CENTER OF BOLT BEING 1" MINIMUM.
SEE DETAIL (2 / S-4)
8. WALL CROSS BRACING SHALL CONSIST OF (1) DIAGONAL CS16 STRAP IN EACH DIRECTION W/ (3) SIMPSON FPHSD #10 SCREWS. STRAPS SHALL BE CONNECTED FROM COLUMN TO EAVE STRUT.
9. ROOF CROSS BRACING SHALL CONSIST OF (1) DIAGONAL CS16 STRAP IN EACH DIRECTION W/ (3) SIMPSON FPHSD #10 SCREWS. STRAPS SHALL BE CONNECTED FROM EAVE STRUT TO EAVE STRUT.

GIRDER / PURLIN LIVE LOAD NOTE

IF WORK IS EVER REQUIRED TO BE CONDUCTED ON ROOF, THE CONTRACTOR SHALL PROVIDE TEMPORARY MID-SPAN SUPPORT AT ALL GIRDERS AND PURLINS. TEMPORARY SUPPORT SHALL NOT BE REMOVED UNTIL ALL ROOF WORK HAS BEEN COMPLETED.

14/

Exhibit #4

**Emergency Fire Safety Plan
Rocky Canyon Dog Kennel (RCK)
6803 Rocky Canyon Rd
Atascadero, Ca
Owners - Melanie Tucker**

At Rocky Canyon Kennel it is our job to make sure the dogs in our care are happy and safe, give their families of peace of mind while they are on vacation.

Call; 911

Cal – Fire Emergency number (805)238-2211
Atascadero Fire Department 805-461-5051

Rocky Canyon facilities, metal siding – concrete slab

1. All fire safety information posted at drop off point for dogs
2. Dog owners will be notified as soon as dogs are safe!
3. Fire Extinguishers are on site. All Fire exits will be clearly marked. The facility is well equipped with fire extinguishers and the closest fire rescue squad is a few
4. On site 2,500 Gallon Water storage tank; Fire connection outlet valve 2-1/2 brass national standard.
5. We have multiple areas that play a key role in quickly and efficiently evacuating the dogs,”. Grouping the

Exhibit #4

smaller dogs from the larger ones, as well as containing non social dogs, our staff whose single goal isto get the dogs out of harm's way with as little stress and chaos as possible

6. We practicing our escape routes and we will be sure that your pets are part of our organized evacuation plan. We rehearse our plan repeatedly with all dogs. We keep collars on and leashes ready in case we have to evacuate quickly with your pet or firefighters need to rescue your pet.
7. Evacuate all dogs from buildings in to fence service area
- Evacuate all dogs on a leash or in pet carrier, load Dogs into vans to safety area away from fire; Vans equipped to haul many dogs at a time to safety

Emergency Number: Contacts

Melanie Tucker (805) 423-8016

Nancy Lillo (805) 423-8020 - Mother

Aaron Mills (805) 610-2740 (805) 610-1256

Roy McDaniel (805) 462-8739 – Lives on the property

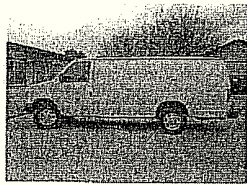
Exhibit #5

Exhibit #5

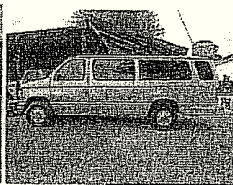
Emergency Flood Plan
 Rocky Canyon Dog Kennel (RCK)
 Emergency Plan –Remove Dogs to Safety
 6803 Rocky Canyon Rd
 Atascadero, Ca
 Owner; Melanie Tucker

It is RCK's responsibility for the Health and safety of
 Animals

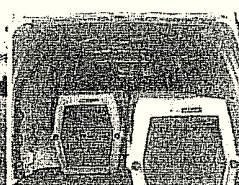
1. History has proven that the, Salinas River does not flood overnight. It takes weeks and sometimes months of rain even get to flooding stage.
2. History has shown through radio, emergency services, Salinas River has a 36 hours notification system alerting the public Salinas will be over flowing and cresting 5:03pm today.
3. If the forecast calls for heavy rain and/or flooding of the Salinas River, the dogs are to be moved to our facility at 9410 Asuncion Rd. All owners will be notified at once.
4. We have Two (2) vans that can each move 10-15 dogs at a time. Our max capacity at RCK is 40 dogs. It would take two trips – we have two (2) vans to evacuate all dogs from RCK. Our facility on Asuncion Rd is large enough to handle the over 70 dogs and can easily be adjusted to handle the extra dogs from RCK in case of an emergency.
5. See Picture below – Vans to move dogs



One of our vans



The other van



Inside the vans

Exhibit #5**Emergency Number: Contacts**

Melanie Tucker (805) 423-8016

Nancy Lillo (805) 423-8020 - Mother

Aaron Mills (805) 610-2740 (805) 610-1256

Roy McDaniel (805) 462-8739 – Lives on the property

Exhibit #6

Emergency Flooding Plan
Building Dis-Assembly
Rocky Canyon Dog Kennel (RCK)
6803 Rocky Canyon Rd
Atascadero, Ca
Owner; Melanie Tucker

In Case of Heavy Rain and/or Flooding;

1. Safety Plan has been put in place and determination that the granting of the variances will not result in increased flood heights; Internal alterations will not result in an increase in flood heights
2. RCK at the highest point of dog kennel is less than 3 ft in flood-way.
3. History has shown through radio, emergency, Salinas River has a 24 hours notification system alerting the public about the river cresting.
4. History; 1983 Klaus Heilmann created the legal parcel – built house on property in 1983 – 2013 – as of 30 years later that property has never flooded;
5. History has proven that the, Salinas River does not flood overnight. It takes weeks and sometime months of rain even get to flood stage.
6. Emergency Plan Solutions: Option of removing our building from Rocky Canyon Road in the event of a potential flood. Aaron Mills Emergency contact number where he can be reached at (Number) Aaron Mills is a metal building contractor and has stated that he would be able to dis-assembly the siding of the building and remove them from the premises in less than a 8 hours period.
Mr. Mills has crews of several experienced workers that would Assist in the remove metal siding and allow the water to run freely through the open space. Mr. Mills has portable generators in case of a power outage to run equipment. Mr. Mills has trailers, trucks and all the necessary equipment to complete the task.
7. Even though our building is less than 3 feet in the flood zone and complete removal would not be necessary; RCK is offering this solution to continue our business for our valuable customers;

Exhibit #6**Emergency Number: Contacts**

Melanie Tucker (805) 423-8016

Nancy Lillo (805) 423-8020 - Mother

Aaron Mills (805) 610-2740 (805) 610-1256

Roy McDaniel (805) 462-8739 – Lives on the property



CAL FIRE
San Luis Obispo
County Fire Department



Robert Lewin, Fire Chief

Exhibit # 9

635 N. Santa Rosa • San Luis Obispo, CA 93405
Phone: 805-543-4244 • Fax: 805-543-4248
www.calfireslo.org

211

April 23, 2013

Rocky Canyon Kennels
Robert Tucker
9400 Asuncion Road
Atascadero, CA. 93422

Subject: Rocky Canyon Kennels – Fire Safety Plan Addendum (PMT2011-01817)

Mr. and Mrs. Tucker,

As a result of the recent onsite inspection, CAL FIRE/San Luis Obispo County Fire Department has made the following addendum to the Fire Safety Plan provided on August 13, 2012.

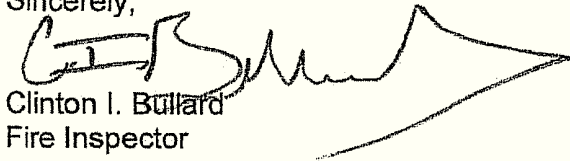
- A fire sprinkler system shall not be required to be installed within the dog kennel/metal enclosure structure.

This determination was made based upon the following circumstances –

1. The existing kennel operation and associated metal enclosure pre-date the current 1,000 square foot fire sprinkler ordinance.
2. The general public is not allowed routine access to the dog kennel/metal enclosure.
3. Based upon the design and construction of the dog kennel/metal enclosure, the installation of a code compliant fire sprinkler system would present significant challenges.
4. The building materials for the dog kennel/metal enclosure are of a non-combustible and/or non-flammable nature.

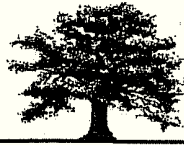
All remaining requirements set forth within the Fire Safety Plan must be satisfied prior to this department conducting a final inspection.

Sincerely,


Clinton I. Bullard
Fire Inspector

Exhib, T # 11

22/



License #0554020

Ted Hamm Insurance Services

Crop Insurance • Farm • Ranch • Home • Auto • Health • Life • Worker's Compensation

June 3, 2013

To Whom It May Concern:

The subject property at Rocky Canyon Road is eligible for flood insurance. The elevation certification will be used to establish the rate and acceptance. In preliminary checking with the flood carriers it is insurable

Thank you

Ted Hamm